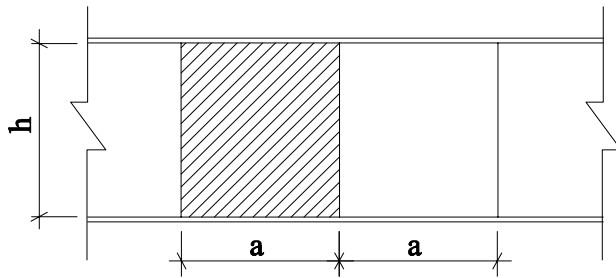




Design of Interior Panel of Built-Up Girder with Transverse Stiffeners Subjected to Shear Force



Materials

Grade:	SEL("Material/ASTM"; NAME;)	=	A36
F_y =	TAB("Material/ASTM"; F_y ;NAME=Grade)	=	36 ksi
E =			29000 ksi

Loads

The required shear strength at the start of this panel from the end , V_u :

V_u =			120 kips
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Section Details

Depth, d =			36 in
Web thickness, t_w =			0.3125 in
Clear distance between stiffeners, a =			99 in
Upper flange width, b_{fc} =			12 in
Upper flange thickness, t_{fc} =			1.50 in
Lower flange width, b_{ft} =			12.00 in
Lower flange thickness, t_{ft} =			1.50 in

Shear Strength for this Panel

h =	$d-t_{fc}-t_{ft}$	=	33.00 in
ψ =	a/h	=	3.00
k_{v1} =	$5+(5/\psi^2)$	=	5.56

Based on AISC Specification Section G2.1, $k_v=5$ when $a/h>3$ or $a/h>[260/(h/t_w)]^2$

λ_w =	h/t_w	=	106
ψ_1 =	$(260/\lambda_w)^2$	=	6.02
Use k_v =	IF($\psi>3$ AND $\psi>\psi_1$;5; k_{v1})	=	5.56
λ_{w1} =	$1.10*\sqrt{(k_v*E/F_y)}$	=	74
λ_{w2} =	$1.37*\sqrt{(k_v*E/F_y)}$	=	92

Calculate C_v according to Eqns. G2-2, G2-3, G2-4 and G2-5:

C_{v1} =	IF($\lambda_w \leq \lambda_{w1}$;1;IF($\lambda_w > \lambda_{w1}$ AND $\lambda_w \leq \lambda_{w2}$; λ_{w1}/λ_w ; $1.51*k_v*E/(F_y*\lambda_w^2)$))=		0.60
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Check the additional limits from AISC Specification Section G3.1 for the use of tension field action:

$$A_w = d \cdot t_w = 11.3 \text{ in}^2$$

$$\eta = \frac{A_w \cdot 2}{b_{fc} \cdot t_{fc} + b_{ft} \cdot t_{ft}} = 0.63$$

$$v_{fc} = \frac{h}{b_{fc}} = 2.75$$

$$v_{ft} = \frac{h}{b_{ft}} = 2.75$$

$$\text{Check_TF} = \text{IF}(\psi > \text{MIN}(3; \psi_1) \text{ AND } \eta > 2.5 \text{ AND } v_{fc} > 6 \text{ AND } v_{ft} > 6; "1"; "2") = 2$$

$$\text{Case} = \text{IF}(\text{Check_TF} = "1"; "NP"; "P") = P$$

"P": permitted and "NP": not permitted

Calculate C_v and V_n , according to Section G3-2a and b:

$$C_{v2} = \text{IF}(\lambda_w \leq \lambda_{w1}; 1; (C_{v1} + \frac{1 - C_{v1}}{1.15 \cdot \sqrt{1 + (\frac{a}{h})^2}})) = 0.71$$

Nominal shear strength:

$$V_n = \text{IF}(\text{Case} = "PERMITTED"; 0.6 \cdot F_y \cdot A_w \cdot C_{v2}; 0.6 \cdot F_y \cdot A_w \cdot C_{v1}) = 146 \text{ kips}$$

$$\Phi_v = 0.90$$

$$\Phi_v V_n = \Phi_v \cdot V_n = 131 \text{ kips}$$

$$\text{Shear_safety} = \text{IF}(\Phi_v \cdot V_n > V_u; "Safe"; "Unsafe") = \text{Safe}$$

$$\text{Force_ratio} = V_u / \Phi_v V_n = 0.92$$

Design Summary

$$V_u = 120 \text{ kips}$$

$$\Phi_v V_n = \Phi_v \cdot V_n = 131 \text{ kips}$$

$$\text{Force_ratio} = V_u / \Phi_v V_n = 0.92$$

$$\text{Shear_safety} = \text{IF}(\Phi_v \cdot V_n > V_u; "Safe"; "Unsafe") = \text{Safe}$$