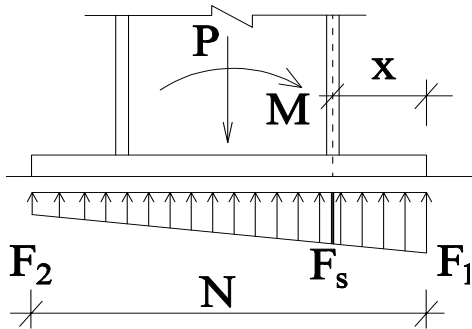




Design of Base Plate Bearing on Concrete Subjected to Small Eccentricity, $e \leq N/6$



Loads

Dead Load, P_D =		50 kips
Live Load, P_L =		90 kips
Moment from D.L., M_D =		100 kip*in
Moment from L.L., M_L =		180 kip*in

Base Plate Material Properties

Grade:	SEL("Material/ASTM"; NAME;)	=	A36
Yield stress, f_{yp} =	TAB("Material/ASTM"; F_y ;NAME=Grade)	=	36 ksi

Column, Base Plate and Pedestal Dimensions

Concrete strength for pedestal (f'_c):

f'_c =		3 ksi	
Sec.:	SEL("AISC/W"; NAME;)	=	W10X112
Pedestal depth, P_d =		17 in	
Pedestal width, P_w =		14 in	
Base plate depth, N =		17 in	
Base plate width, B =		14 in	
Depth of column, d =	TAB("AISC/W"; d ; NAME=Sec.)	=	11.4 in
Flange of column, b_f =	TAB("AISC/W"; b_f ;NAME=Sec.)	=	10.4 in

Check Eccentricity Size

M_t =	M_D+M_L	=	280 kip*in
P_t =	P_D+P_L	=	140 kips
e =	M_t/P_t	=	2.00 in
Check_e=	IF($e \leq N/6$;"O.K."; "not O.K.")	=	O.K.

(Note that: if this check was not O.K., this template will give a wrong solution)

The Ultimate Load and Moment

(Chapter 2 of ASCE/SEI 7)

P_u =	$1.2*P_D+1.6*P_L$	=	204 kips
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$$M_u = 1.2 * M_D + 1.6 * M_L = 408 \text{ kip}\cdot\text{in}$$

The Maximum Bearing Stress, F_b

$$\Phi_c = 0.60$$

$$A_1 = N * B = 238.00 \text{ in}^2$$

$$A_2 = P_d * P_w = 238.00 \text{ in}^2$$

$$F_{b1} = 0.85 * \Phi_c * f_c * \sqrt{A_2 / A_1} = 1.53 \text{ ksi}$$

$$F_{b2} = 1.7 * f_c = 5.10 \text{ ksi}$$

$$F_b = \text{MIN}(F_{b1}; F_{b2}) = 1.53 \text{ ksi}$$

$$F_1 = \text{IF}(\text{Check_e} = \text{"O.K."}; \frac{P_u}{A_1} + \frac{M_u * N / 2}{(B * N^3) / 12}; \text{" "}) = 1.46 \text{ ksi}$$

$$F_2 = \frac{P_u}{A_1} - \frac{M_u * \frac{N}{2}}{B * N^3 / 12} = 0.25 \text{ ksi}$$

$$\text{Check_F} = \text{IF}(F_1 < F_b; \text{"Safe"}; \text{"Unsafe"}) = \text{Safe}$$

Base Plate Thickness:

The critical section is at distance (x) from the plate edge, where:

$$x = \frac{N - 0.95 * d}{2} = 3.09 \text{ in}$$

The distance from the base center to this section (x_1) and the stress at this section (F_s) can be calculated as:

$$x_1 = \frac{N}{2} - x = 5.41 \text{ in}$$

$$F_s = \text{IF}(\text{Check_e} = \text{"O.K."}; \frac{P_u}{A_1} + \frac{M_u * x_1}{(B * N^3) / 12}; \text{" "}) = 1.24 \text{ ksi}$$

The factored moment for a 1-in strip at this section (M_s) can be calculated as follows:

$$M_s = \frac{F_s * x^2}{2} + (F_1 - F_s) * \frac{x^2 * 0.67}{2} = 6.62 \text{ kip}\cdot\text{in}$$

$$t_{p,\text{min}} = \sqrt{\frac{4 * M_s}{0.9 * f_{yp}}} = 0.90 \text{ in}$$

Summary: Use Plate with The Following Minimum Dimensions

$$\text{Plate length} = N = 17 \text{ in}$$

$$\text{Plate width} = B = 14 \text{ in}$$

$$\text{Plate thickness} = t_{p,\text{min}} = 0.90 \text{ in}$$